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FRENCH RIVER BASIN CHARLTON, MASSACHUSETTS



LITTLE NUGGET LAKE DAM MA 00103

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

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DEPARTMENT OF THE ARMY NEW ENGLAND DIVISION, CORPS OF ENGINEERS WALTHAM, MASS. 02154

OCTOBER 1978

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number)

Little Nugget Lake Dam is a dry-stone masonry and earth dam with a broad crested weir spillway. The dam has a maximum height of 13 feet and is approximately 170 feet long including spillway length. It has been classified as a "significant" hazard. The dam is considered to be in poor condition. An outflow test flood of 1,970 cfs (½ the PMF) will overtop the west dike by about 2.2 feet and the dam by 1.0 foot and the swale by 2.9 feet.



DEPARTMENT OF THE ARMY

NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02154

REPLY TO ATTENTION OF:

NEDED

JAN 3 0 1979

Honorable Edward J. King Governor of the Commonwealth of Massachusetts State House Boston, Massachusetts 02133

Dear Governor King:

I am forwarding to you a copy of the Little Nugget Lake Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Environmental Quality Engineering, the cooperating agency for the Commonwealth of Massachusetts. In addition, a copy of the report has also been furnished the owner, the Town of Charlton, Board of Selectmen, Town Hall, Main Street, Charlton, Massachusetts 01507, ATTN: Mr. Leonard Haebler, Chairman.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

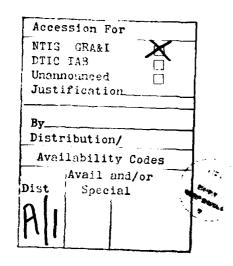
I wish to take this opportunity to thank you and the Department of Environmental Quality Engineering for your cooperation in carrying out this program.

Sincerely yours,

Incl As stated

Colonel, Corps of Engineers

Division Engineer



MA 00103

FRENCH RIVER BASIN CHARLTON, MASSACHUSETTS

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

NATIONAL DAM INSPECTION PROGRAM

PHASE I INSPECTION REPORT

BRIEF ASSESSMENT

Identification No.: MA00103

Name of Dam: Little Nugget Lake

Town: Charlton

County and State: Worcester County, Massachusetts

Stream: Little River - A tributary of French River

Date of Inspection: August 31, 1978

Little Nugget Lake Dam is a dry-stone masonry and earth dam with a broad crested weir spillway. The existing dam was constructed prior to 1924 with major repairs performed in 1940 and 1967-1968. The dam has a maximum height of 13 feet and is approximately 170 feet long including spillway length. A 30-foot long swale located on the east end of the dam is about 2 feet lower than the remainder of the dam. Most of the downstream slope of the dam is a vertical dry-stone masonry wall. A 3-foot high dike is located several hundred feet west of the dam. The 20.5 foot long spillway is constructed of concrete with 2.5-foot high mortared stone masonry training walls.

Little Nugget Lake Dam was neither designed nor constructed according to current approved state-of-the-art procedures. Based upon the visual inspection at the site, the limited engineering data, and little evidence of operational or maintenance procedures, it was concluded that there are deficiencies that must be corrected to assure the continued performance of this dam. Generally, Little Nugget Lake Dam is considered to be in poor condition. Because of the potential danger to lives and property downstream Little Nugget Lake Dam has been classified as a "significant" hazard.

The following visible signs of distress indicate a potential hazard at the site: bulging of the stone wall on the downstream face of the dam, slight-to-moderate seepage at the downstream toe of the dam, accumulation of debris in the spillway channel; inoperable slide gate; spalling concrete on the spillway and missing concrete on the end of the spillway; lack of adequate riprap protection on the upstream face of the dam; and thick vegetation growing immediately downstream of the dam.

Hydraulic analyses indicate that the spillway (without flashboards) and emergency spillway can discharge a flow of 300 cubic feet per second (cfs) when the water surface is at elevation (El) 771.5, which is the average elevation of the crest on the dike west of the dam. An outflow test flood of 1,970 cfs (one-half the probable maximum flood) will overtop the west dike by about 2.2 feet and the main dam by 1.0 foot and the swale by 2.9 feet. The spillways may be inadequate since they can discharge only 15 percent of the test flood before the dam is overtopped. In the event of overtopping, complete failure of the dam and west dike could occur.

It is recommended that the Owner employ a qualified consultant to evaluate the stability of the dam, conduct a more detailed hydrologic and hydraulic investigation, and evaluate the seepage underneath the dam. It is also recommended that the Owner repair the concrete on the weir; repair the gate operating mechanism; add riprap to the upstream slope of the dam and dike; and remove all trees and brush from the upstream face and downstream of the toe of the dam. The Owner should also implement a systematic program of inspection and maintenance.

The above recommendations and remedial measures should be implemented by the Owner within a period of one year after receipt of this Phase I Inspection Report. An alternative to these recommendations would be draining the pond and breaching or removing the dam,



Edward M. Greco, P.E. Project Manager

Metcalf & Eddy, Inc.

Connecticut Registration No. 08365

Approved by:

Stephen Z. Bishop, P.E.

Vice President Metcalf & Eddy, Inc.

Massachusetts Registration No. 19703



This Phase I Inspection Report on Little Nugget Lake Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

RICHARD F. DOHERTY, MEMBER

Water Control Branch Engineering Division

JOSEPH A. MCELROY, MEMBER

JOSEPH A. MCELROY, MEMBER Foundation & Materials Branch Engineering Division

CARNEY M. TERZIAN, CHAIRMAN

Chief, Structural Section

Design Branch

Engineering Division

APPROVAL RECOMMENDED:

JOE B. FRYAR

Chief, Engineering Division

PREFACE

This report is prepared under guidance contained in Recommended Guidelines for Safety Inspection of Dams, for a Phase I Investigation. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D. C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test Flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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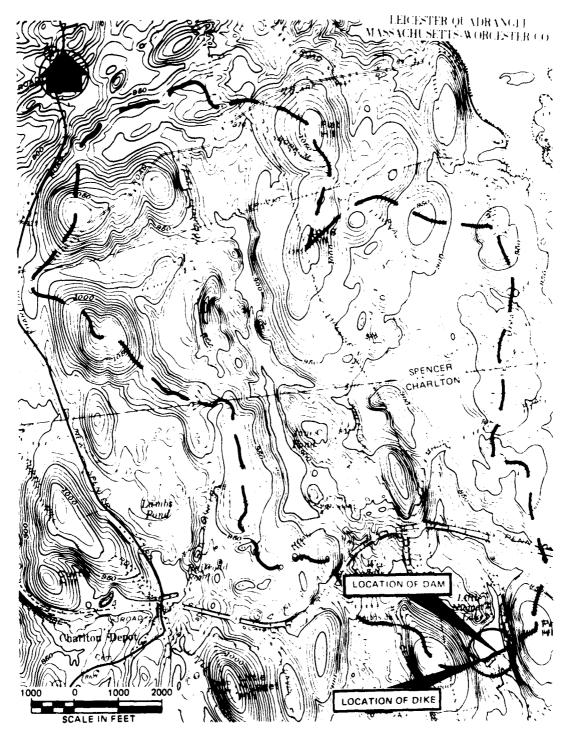
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OVERVIEW LITTLE NUGGETT LAKE CHARLTON, MASSACHUSETTS



VIEW FROM UPSTREAM OF EAST ABUTMENT

Location and Direction of Photographs Shown on Figure in Appendix B



LOCATION MAP - LITTLE NUGGET LAKE DAM

NATIONAL DAM INSPECTION PROGRAM

PHASE I INSPECTION REPORT

LITTLE NUGGET LAKE

SECTION 1

PROJECT INFORMATION

1.1 General

a. Authority. Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a national program of dam inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Metcalf & Eddy, Inc. has been retained by the New England Division to inspect and report on selected dams in the State of Massachusetts. Authorization and notice to proceed was issued to Metcalf & Eddy, Inc. under a letter of July 28, 1978, from Ralph T. Garver, Colonel, Corps of Engineers. Contract No. DACW 33-78-C-0306 has been assigned by the Corps of Engineers for this work.

b. Purpose:

- (1) Perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.
- (2) Encourage and assist the States to initiate quickly effective dam safety programs for non-Federal dams.
- (3) Update, verify and complete the National Inventory of Dams.

1.2 Description of Project

- a. Location. The dam is located on Little River, a tributary of the French River, Town of Charlton, Wordester County, Massachusetts (see Location Map and Figure B-1). A small dike is located west of the dam (see Location Map and Figure B-2).
- Description of Dam and Appurtenances. Little Nugget Lake Dam is an earthfill dam with a downstream vertical stone face. There are no drawings available for this dam. A concrete spillway with mortared stone masonry training walls is located on the right abutment. The dam and spillway are about 170 feet long and the dam has a maximum height of 13 feet. The spillway is 20.5 feet wide and the downstream end of the concrete apron is partially missing. Flashboards 1.15 feet high are mounted on the crest. Sidewalls of the spillway are constructed of mortared stone and are 2.6 feet (west side) to 2.5 feet (east side) above the crest. The spillway discharges into a channel which connects with a separate smaller channel formed by water seepage apparently from underneath the dam.

The crest of the earthfill embankment is relatively flat for most of its length, at about elevation (El) 772.5. A swale is located for a length of about 30 feet on the east crest of the dam. The low point of the swale is at El 770.8. The swale serves as an emergency spillway. The axis of the dam is slightly concave upstream. The upstream face of the dam contains large riprap stones which may have been a vertical face at one time. The downstream vertical face is a dry masonry stone wall which bulges in the downstream direction. Voids as deep as ? feet into the embankment were commonly observed in the joints. From the spillway to a point 15 feet east of the spillway there is a partly + roded knoll and no vertical stone face. There is a large pool of relatively clear water at the toe of the dam.

The one known outlet structure for the dam is located near the center (see Figure B-1). The intake consists of a dry stone sluiceway surrounded by riprap. A threaded valve stem extends through a metal plate resting on top of the sluiceway. There is no mechanism at the site for opening the gate. The invert of this outlet is at El 760.1. The vertical stone wall face at the outlet end is mortared for a width of 5 feet and a height of 5.3 feet. A leak of approximately 1 to 2 gpm of clear water was discharging from the outlet pipe. There are sections of an old deteriorated pipe downstream of the dam and outlet pipe.

A dike is located west of the dam. The dike is 81 feet long and has a maximum height of 3 feet. The dike is an earthfill embankment with a vertical downstream stone wall which has deteriorated.

- c. Size Classification. Little Nugget Lake Dam is classified in the "small" category since it has a maximum height of 13 feet and a maximum storage capacity of 109 acre-feet.
- d. Hazard Classification. Downstream of the dam are two homes, three culverts for a local road, and bridges for the Northside Turnpike and the Massachusetts Turnpike. In the event of overtopping and complete failure of the dam, a few lives could be lost and appreciable property damage could occur. Accordingly the dam has been classified in the "significant" hazard category.
- e. Ownership. The dam and dike are located on property owned by the Town of Charlton, Massachusetts, Board of Selectmen, Town Hall, Main Street, Charlton, Massachusetts C1507. Mr. Leonard Haebler, Chairman, Board of Selectmen (617-248-5668) granted permission to enter the property and inspect the dam and dike. Frior to 1957 the dam was owned by Little Nugget Lake, Inc.
- f. Operators. The only known operator of this dam is the Town Highway Superintendent. There are no fences around the outlet structure.

- g. Purpose of Dam. The earliest records in 1924 indicate the dam provided storage water for unknown purposes. The lake and dam now serve for recreation and flood control.
- h. Design and Construction History. There are no drawings available that show the construction of the dam prior to 1924. At that time the dam was owned by Mr. George M. Wright. Ownership was transferred to Mr. Edwin H. Perry in 1930, then to the Federal Land Bank in 1939, and to Isiah and Annette Newman in 1945. From 1948 to 1957 the dam was the property of Little Nugget Lake, Inc.

Previous inspection reports indicate no leakage in 1924. Seepage was then observed in 1930, 1938, and from 1964, until repairs were made in 1967. Holes and depressions were observed on the embankment crest in 1924, 1930, 1938, 1940, 1954, 1955, 1956, 1964, 1966, until 1967 repairs. One sinkhole in 1938 was 15 by 3 feet in plan dimensions and 1 foot deep. Sinkholes were again observed in 1973, one at the gate and one at the spillway. Evidence of piping was also noted in 1973.

Evidence of overtopping the dam was noted in 1930, and again in 1933 when sand bags and boards were readied as water came within 6 inches of the top. In 1938 a 6-foot wide by 2-1/2-foot deep break in the east end of the dam was observed. In 1955 the embankment was overtopped for the full length, causing face stones and soil to wash out on the downstream face. The overtopping washed out a road downstream of the dam, reported as the Stafford Turnpike. Flashboards were in place and the gate was inoperable. The water level was observed at the top of the embankment in two low places in 1967 prior to repairs.

In 1940, it is reported that the spillway crest was lowered 2 feet and a core wall placed in the embankment. In 1967 and 1968 the dam was apparently repaired and a new concrete cutoff wall 10 feet long was extended underneath the

existing cutoff wall. The crest was raised to the same elevation as the top of stone abutment walls at spillway. The missing east abutment wall on the spillway was also reported reconstructed and a new gate was placed in the outlet pipe.

i. Normal Operating Procedure. There are no operating procedures at the dam. There is one known outlet structure controlled by a slide gate located near the center of the dam. The outlet is reported to be operable, but has not been opened in four years. At that time vandals threw rocks into the inlet, so it was necessary to clean out the stones to close the gate. There is no handwheel on the valve stem. The stem appears to have a removable protective cap.

The spillway for Little Number lave law to ungated. Flashboards extend gords: the full width of the spillway. Plashboards are nemoved from the spillway to new late the water level. The orenator ineffect in the law the lake water level similarity in the law that ore used for water suitly.

1.3 Pertinent Data

- a. Drainage Area. The drainage the firstitle Nugget Lake is estimated to test, et acres (3.05 square miles). Little Fiver flows from the north through rural, sparsely revelored woodland and swamp. Restiential development is limited to the few homes located north and east of the lake.
- b. Discharge at the Dam Cite. Incontrolled discharge at the dam site flows over the Confect long concrete-faced broad-crested wein at the west end of the dam and into the stream channel. The channel is approximately Confect wide 50 feet downstream of the edge of the spillway. The spillway surface is concrete lined and covered mostly with asphalt. The downstream slope is spalled, and the end of

the concrete apron is missing. The channel is bounded by trees, and contains many stones and debris. A pool of water at the toe of the slope in the center of the main dam discharges into a separate channel which joins with the main channel approximately 200 feet downstream of the dam. Water discharging through the 18-inch diameter outlet in the downstream face of the dam contributes to the pool of standing water. The channel flows through three culverts under a road and then further downstream under the Northside Turnpike and the Massachusetts Turnpike before it discharges into Pikes Pond. Below Pikes Pond Little River flows through sparsely developed woodland. The next major population center is the Town of Webster, almost 5 miles downstream of Pikes Pond.

Hydraulic analyses indicate that the spillway (without flashboards) and emergency spillway can discharge an estimated 300 cfs when the water surface is at El 771.5, which is the average elevation of the crest of the dike, and 150 cfs when the water surface is at El 770.8, which is the lowest point in the east swale on the main dam. This swale serves as an emergency spillway.

An inflow test flood of 2,060 cfs (one-half the probable maximum flood) adjusted for surcharge storage results in a maximum discharge of 1,970 cfs. This outflow will overtop the main dam by 1.0 foot, the swale by 2.9 feet and the dike by 2.2 feet. In 1930 and 1933 there was evidence of overtopping. Records revealed the dam was again overtopped in 1955.

- c. Elevation (feet above MSL (Mean Sea Level)).

 A benchmark was established at El 769.0 on the crest of the spillway. This elevation was estimated from a United States Geological Survey (USGS) topographic map.
 - (1) Top dam Main dam: 770.8 to 772.9 - Dike section: 771.0 to 772.2
 - (2) Test flood pool: 773.7

- (3) Design surcharge (original design): Unknown
- (4) Full flood control pool: N/A
- (5) Recreation pool: 769.0
- (6) Spillway crest (ungated): 768.9 to 769.0
- (7) Upstream portal invert diversion tunnel: N/A
- (8) Stream bed at centerline of dam: 761.0
- (9) Tailwater: N/A (water in downstream channel) Tailwater: 759.4 (water in pool downstream of main dam)

d. Reservoir

- (1) Length of maximum pool: 1,500 feet
- (2) Length of recreation pool: 1,500 feet
- (3) Length of flood control pool: N/A

e. Storage (acre-feet)

- (1) Test flood surcharge: 60 at El 773.7
- (2) Top of dam: 109
- (3) Flood control pool: N/A
- (4) Recreation pool: 61 (Approximate)
- (5) Spillway crest: 61

f. Reservoir Surface (acres)

- *(1) Top dam: 12.8
- *(2) Test flood pool: 12.8

^{*}Based on the assumption that the surface area will not significantly increase with changes in pond elevation from 769.0 to 773.7.

- (3) Flood-control pool: N/A
- (4) Recreation pool: 12.8
- (5) Spillway crest: 12.8

g. Dam

- (1) Type Main dam: dry-stone masonry earthfill Dike section: dry-stone masonry earthfill
- (2) Length Main dam: 170 feet - Dike section: 81 feet
- (3) Height Main dam: (maximum) 13 feet
 Dike section: (maximum) 3 feet
- (4) Top width Main dam: 22 feet
 Dike section: 7 feet
- (5) Side slopes Main dam: downstream vertical to 0.25:1; upstream not visible
 Dike section: upstream 4:1; downstream vertical
- (6) Zoning: Unknown
- (7) Impervious core: Unknown
- (8) Cutoff: Poured in two sections 24 feet long at top and 10 feet long at bottom stepped reinforced concrete wall; short section of sheeting in 1938
- (9) Grout curtain: Unknown.

i. Spillway

- (1) Type: Broad crest
- (2) Crest length: 20.5 feet
- (3) Crest elevation: East 769.0 (assumed benchmark)
 West 768.9

- (4) Gates: None, flashboards 1.15 feet high
- (5) Upstream channel: Mortared masonry sidewalls at spillway, concrete slab covered with asphalt
- (6) Downstream channel: Downstream face of weir is spalled and bottom is missing.

 Below missing bottom is natural channel bounded by rocks. This channel converges with the channel created by seepage about 200 feet downstream of dam.
- j. Regulating Outlets. The only regulating outlet at this dam is the one 3- by 3-foot sluice gate located near the center of the dam. The outlet has a capacity of 32 cfs (10.5 cfs per square mile). The gate has not been operated in about 4 years, and parts of the opening mechanism are missing.

SECTION 2

ENGINEERING DATA

2.1 General. There are no drawings available of Little Nugget Lake Dam. Visual observations during inspection, review of previous inspection reports, and conversations with the Owner and with personnel from Town, State and County agencies provided the remainder of the data for this evaluation.

We acknowledge the assistance and cooperation of personnel of the Massachusetts Department of Public Works, Messrs. Willis Regan and Raymond Rochford, and personnel of the Massachusetts Department of Environmental Quality Engineering, Division of Waterways, Messrs. John J. Hannon and Joseph Iagallo.

Also, we acknowledge the cooperation and assistance of personnel from the Worcester County Engineer's Office, Messrs. John O'Toole, Joseph Brazauskas, and Mr. Wallace Lindquist - recently retired from county service.

Mr. Leonard Haebler, Chairman, Board of Selectmen for the Town of Charlton, granted permission to enter the property and inspect the dam.

- 2.2 <u>Construction Records</u>. There are no as-built drawings for the dam. Information on the cutoff wall was obtained from an early County inspection report.
- 2.3 Operating Records. No operating records are available for the dam and no daily record is kept of the elevation of the pool or rainfall at the dam site.

2.4 Evaluation

- a. Availability. Due to the age of this dam, there is no engineering data available.
- b. Adequacy. The lack of in-depth engineering data did not allow for a definitive review. Therefore the adequacy of this dam could not

be assessed from the standpoint of reviewing design and construction data, but is based primarily on visual inspection, past performance history and engineering judgment.

c. Validity. The limited engineering data available is valid for this assessment.

SECTION 3

VISUAL INSPECTION

3.1 Findings

- a. General. The Phase I Inspection of the dam and dike at Little Nugget Lake was performed on August 31, 1978. A copy of the inspection check list is in Appendix A. Periodic inspections of this dam have been made by others since 1924. A partial listing of these inspections is in Appendix B. An inspection by the Massachusetts Department of Public Works was made in 1972 and 1973, and a copy of that report is also included in Appendix B. In addition, earlier inspection reports were reviewed at the Worcester County Engineer's office.
- b. Dam. In general the dam is in poor condition. The downstream vertical stone face has bulged outward between the knoll and the outlet pipe; and the unmortared joints between many of the stones indicate voids back into the earthen embankment. A large pool of relatively clear water immediately downstream of the dam indicates considerable seepage through or underneath the dam. There is a surface depression near the outlet structure west of the swale. A flat filled area is exposed on the northwest corner of the dam next to the spillway.

The area at the downstream toe is completely covered with brush and trees. There is slight to moderate riprap protection of the upstream slope.

c. Appurtenant Structures. The concrete and stonework of the spillway are in poor to fair condition. Pieces of the downstream end of the mortared stone masonry east training wall are dislodged. Some of the mortar of the spillway sidewalls is missing or has cracked. At the time of the inspection, water was flowing over

the flashboards. The concrete facing of the spillway is eroded away, and there are local areas of spalled concrete. Evidence of slight to moderate seepage was observed at the downstream edge of the apron where surface concrete has been completely eroded, exposing underlying stones. The subsided ground surface adjacent to the west spillway sidewall has been filled in with leaves. The downstream channel is relatively clear except for a few overhanging trees and small tree trunks and logs in the bottom of the channel.

The outlet structure is in fair condition. The inlet is not visible. The inlet to the gate structure is a dry stone masonry wall. A gate stem extends through a metal welded platform mounted on top of the masonry walls. The wheel is missing and the gate is reportedly 3 feet by 3 feet. The gate stem is reportedly operable. The gate was last opened four years ago. The Owner indicated that vandals threw rocks into the inlet and it was necessary to clean out the rocks to close the gate. For that reason the reservoir surface is now regulated by removing flashboards from the spillway.

The outlet structure is in good condition. The outlet from the gate structure is an 18-inch diameter corrugated metal pipe recessed into a mortared vertical stone wall which blends into the unmortared downstream stone wall. There is leakage from the outlet pipe estimated at 1 gpm. A small deflector stonewall extends 4.5 feet downstream of the outlet pipe.

- d. Reservoir Area. Moderately populated areas of residential development are located on the east, north and south sides of the lake.
- e. Downstream Channel. The discharge from the spillway flows down Little River before it flows into Pikes Pond. About 1,000 feet downstream of the dam, the Little River crosses under a road through three culverts with diameters of 3 feet, 3 feet and 2.5 feet.

3.2 Evaluation. The above findings indicate that the dam has several areas of distress that require attention. It is evident that the dam is not adequately maintained and that deterioration will continue unless action is taken. Recommended measures to improve these conditions are included in Section 7.

SECTION 4

OPERATING PROCEDURES

- 4.1 Procedures. There are no operating procedures at Little Nugget Lake Dam.
- 4.2 Maintenance of Dam. There is no systematic maintenance program at the dam.
- 4.3 Maintenance of Operating Facilities. The outlet gate is closed and cannot be opened with the existing available mechanism. Flow over the spillway is controlled only to the extent that flashboards are removed.
- 4.4 Description of Any Warning System in Effect.
 There is no warning system in effect at the dam.
- Evaluation. Little Nugget Lake Dam is in poor condition and has been placed in the "significant" hazard category because of the possible danger to life and property downstream. For this reason, it is important that procedures for operation, maintenance, and emergencies be implemented as recommended in Section 7.

SECTION 5

HYDRAULIC/HYDROLOGIC

5.1 Evaluation of Features

Design Data. The Probable Maximum Flood а. (PMF) rate was determined to be 1,350 cfs per square mile. This calculation is based on the average drainage area slope of 2.5 percent, the pond-plus-swamp area to drainage area ratio of 13 percent, and the U.S. Army Corps of Engineers' guide curves for Maximum Probable Flood Peak Flow Rates (dated December 1977). Applying one-half the PMF to the 3.05 square miles of drainage area results in a calculated peak flood of 2,060 cfs as the inflow test flood. By adjusting the inflow test flood for surcharge storage, the maximum discharge rate was established as 1.970 cfs, with a water surface at El 773.7.

Flow over the crest of the dam and the dike is predicted to be 1,060 cfs, and flow through the spillway (without flashboards) and emergency spillway (swale area on main dam) would be 910 cfs. The maximum head on the dam would be 1.0 feet, and 2.9 feet in the swale area with a discharge of 382 cfs per foot of width. Depth at critical flow would be at 0.60 feet with a velocity of 4.35 feet per second.

Flow over the crest of the dike is predicted to be 666 cfs. The maximum head on the dike is 2.2 feet with a discharge of 8.32 cfs per foot of width. Depth at critical flow would be at 1.29 feet with a velocity of 6.43 feet per second.

Hydraulic analyses indicate that the spillway (without flashboards) and emergency spillway (the swale) can discharge an estimated 300 cfs when the water surface is at El 771.5, which is the average elevation of the crest of the dike. The spillways can discharge only 15 percent of the test outflow before the dam is overtopped.

- b. Experience Data. Hydraulic records are not available for this dam, however, information supplied by the County indicated there was evidence of overtopping the dam in 1930 and 1933. In 1955 the embankment was evertopied for the full length.
- c. Visual Observations. Discharge from Little Nugget Lake is over the spillway located on the west end of the main dam. The spillway is a 20.5-foot long broad crested weir.

The concrete spillway is in poor to fair condition; it is severely pitted and the downstream edge is missing. The west end of the weir is 0.1 foot lower than the east end.

d. Overtopping Potential. Overtopping of the dam by about 1.0 foot, overtopping of the swale by about 2.9 feet, and overtopping of the dike by about 2.2 feet is expected under the outflow test flood of 1,970 cfs. In the event of overtopping, complete failure of the dam and dike could occur.

Failure of the dam would produce a peak discharge of 3,400 cfs and a flood wave 6.5 feet high, at a point 700 feet downstream of the dam, as estimated using the Corps of Engineers criteria.

SECTION 6

STRUCTURAL STABILITY

6.1 Evaluation of Structural Stability

a. <u>Visual Observations</u>. The evaluation of the structural stability of the dam is based on the visual inspection conducted on August 31, 1978.

Based on the observations as discussed in Section 3, Visual Inspection, it is recommended that a more detailed investigation be initiated to evaluate the stability of the dam and the seepage at the downstream toe.

- b. Design and Construction Data. Discussions with the Owner and County and State personnel indicate there are no available plans, specifications, or computations on the design, construction, or repair of the dam. Information on the type, shear strength and permeatility of the soil and/or rock materials is nonexistent.
- c. Operating Records. There is no evidence that any type of instrumentation has ever existed at Little Nugget Lake Dam. The performance of the spillway and dam under prior loading can only be inferred from physical evidence at the site.
- d. Post-construction Changes. The only recorded changes after 1924 were repairs undertaken by the Owner in 1940 and 1967-1968. In 1940 the spillway crest was lowered 2 feet, the apron partly paved with concrete; and a core wall was placed. In 1967 changes included placing concrete on the spillway crest and filling a large hole on the downstream end of the crest; removing the center spillway pier, and placing a 10-foot long concrete cutoff wall. In 1968 changes included replacing the missing downstream end of the east spillway abutment wall, raising the main dam to the same elevation as the top of the stone abutment

walls at the spillway, replacing missing stones on the upstream and the downstream dam face; and replacing the gate in the outlet structure.

e. Seismic Stability. The dam is located in Seismic Zone No. 2 and in accordance with Phase I "Recommended Guidelines" does not warrant seismic analyses.

SECTION 7

ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

7.1 Dam Assessment

Condition. Built prior to 1924, Little Nugget Lake Dam was neither designed nor constructed according to current approved stateof-the-art procedures. Based upon the visual inspection, the limited engineering data. and no evidence of operation or maintenance procedures, it is determined that various deficiencies must be corrected to assure the continued performance of this dam. Generally, the dam is considered to be in poor condition. The principal areas of concern are: seepage at the toe of the dam; outward bulging of the vertical downstream stone wall; deep voids extending into dam through open joints between downstream wall stones; an inoperable slide gate; spalling and missing concrete on the weir; lack of adequate riprap protection on the upstream face of the dam and the dike; erosion caused by surface runoff at the knoll left of the spillway on the downstream slope; vegetation on the upstream slopes of the dam and thick vegetation immediately downstream of the dam; and lack of competent fill behind the west training wall of the spillway.

Hydraulic analyses indicate that the existing spillway (without flashboards) and emergency spillway can discharge a flow of 300 cfs when the water level in the lake is at El 771.5, which is the average elevation of the dike. The spillway may be inadequate since it can discharge only 15 percent of the test flood before the dam is overtopped. An outflow test flood of 1,970 cfs will overtop the dike by 2.2 feet, the main dam by 1.0 foot, and 2.9 feet at the swale.

The limited information available indicates that in 1930, 1933 and 1955 water overtopped the dam.

- b. Adequacy of Information. The lack of in-depth engineering data did not allow for a definitive review. Therefore the adequacy of this dam and west dike could not be assessed from the standpoint of reviewing design and construction data, but is based primarily on visual inspection, past performance history and engineering judgment.
- c. Urgency. The recommendations and remedial measures outlined below should be implemented by the Owner within one year after receipt of this Phase I Inspection Report.
- d. Need for Additional Investigation. Additional investigations to further assess the adequacy of the dam and spillway are outlined below in Section 7.2, Recommendations.
- 7.2 Recommendations. In view of the concerns over the continued performance of the dam, dike and spill-way, it is recommended that the Owner employ a qualified consultant to:
 - a. evaluate the stability of the dam
 - b. conduct a more detailed hydrologic and hydraulic investigation of the site to design an adequate spillway
 - c. evaluate the seepage at the downstream toe of the dam.

The recommendations on repairs and maintenance procedures are outlined below under Section 7.3, Remedial Measures.

7.3 Remedial Measures

- a. Operating and Maintenance Procedures. The dam and spillway are not adequately maintained. It is recommended that the Owner accomplish the following:
 - (1) repair the gate mechanism to make the slide gate operable,
 - (2) repair and replace where missing, the concrete on the weir and replace the missing end of the east training wall

- (3) add riprap on the upstream face of the dam and west dike,
- (4) remove all trees and brush from the upstream face and crest of the dam and the dike,
- (5) remove all trees and brush downstream of the dam,
- (6) institute a definite plan for surveillance and a warning system during periods of unusually heavy rains and/or runoff. The warning system should be coordinated with one at the upstream reservoirs in the watershed, because flooding or failure of the upper dams will have a severe effect on Little Nugget Lake,
- (7) implement a systematic program of maintenance inspections. As a minimum, the inspection program should consist of a monthly inspection of the dam and appurtenances and be supplemented by additional inspections during and after severe storms. All repairs and maintenance should be undertaken in compliance with all applicable State regulations.
- (8) technical inspection of this dam should be continued on an annual frequency,
- (9) remove miscellaneous fill of leaves, etc. outside the west training wall of the spillway and replace with compacted soil, covered by loam, and then seeded.
- (10) clear all debris from the spillway and downstream channel.
- 7.4 Alternatives. An alternative to implementing the recommendations and remedial measures listed above would be to drain the pond and breach or remove the dam. This could be an undesireable alternative, causing the lowering of the groundwater around the lake resulting in a deleterious effect on the local water wells.

APPENDIX A

PERIODIC INSPECTION CHECKLIST

PERIODIC INSPECTION PARTY ORGANIZATION

PROJECT Little Nugget Lake		MATE Hugust 31,	(*) / \$
		TIME 2 CO p. 11	
		WEATHER rain	
		W.S. ELFV. 769.9	75114
PARTY:			
1. Michael Larson	_ ú	Henry Lord	
. Lyle Branagan	7.		
. Susan Pierce	_ 8		**
David Cole	9		- ·
. Frank Sviokla	_ 10		
PROJECT FEATURE		INSERTTEL BY	1.
. Dan		M Larson	
· Spillway		L Brawagan	
••			
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·			
·			
%	······································		
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			ekamanen e eka 10

PERIODIC INSPECTION CHECK LIST

THE VENT Little Nugget Lake	DATE 8-31-78		
ER CENT PROTURE Dam	NAME M Larson		
FIINE Geotechnical	MAME		
AREA EVALUATED	CONDITIONS		
C FUHANKMENT			
Treat Elevation	varies 770.8 to 772.9		
Turrent Fool Elevation	769.9		
Orximum Impoundment to Date	unknown		
Juntace Chacks	none		
Egyement Condition	no pavement - niscollaneaus fill on right crest, grass on left crest. Local erosion to right of swale, behind intake plate		
Mayement or Settlement of Crest	crest fairly flat west of succee		
Dateral Movement	2 main areas: alls face - lett of spalluray ups tace: rock shifted Cither sid of matrice		
Vertical Alignment	bulging in damstream wall, about min-heigh		
Horizontal Alignment	Concave upstream		
Condition at Abutment and at Concrete Structures	earth and rock at left abutment, iniscellaneous fill in spillway grass covered right abutment		
Indications of Movement of Structural Items on Slopes	no structural items		
Trespassing on Slopes	at spillury footpath on right above training wall, on left below alls wall		
Cloughing or Erosion of Slopes or Abutments	Downstream face left of spillury, evidence of prosion or collapse of unit		
Rock Slope Protection - Riprap Failures	Either side of intake: placed ripiep, some pieces inissing Rooks bulging outward never top. Riprap slope 15' either side of intake		
Unusual Movement or Dracking at or near Toes	none visible		
Unusual Embankment or Downstream	wet areus als of toe, scepage pool to right of		
Piping or Boils	none visible, unconfirmed report of boiling at coest to left of spallway		
Foundation Drainage Features	none visible		
Toe Orains	none visitale		
Instrumentation System	nene		

PERIODIC INSPECTION CHECK LIST

PROJECT Little Nugget	- CATE 8-31-78
PROJECT FEATURE dike	NAME M Larson
MESSIPLINE Geotechnical	MAME
AREA EVALUATED	CONDITION
DIKE EMBANKMENT	
rest Elevation	Varies 7710 to 7715
'urrent Pool Elevation	764.9
Maximum Impoundment to Date	unknown
Jurface Cracks	none
Esvement Condition	nla - crest is earth
Movement or Settlement of Crest	irregular surtaci
Lateral Movement	irregular
Vertical Alignment	nja
Horizontal Alignment	nia
Condition at Abutment and at Concrete Structures	no concrete earth a butments the orthogenine ground, overgroun with small trees that we are thouse adjacent to lett abutment
Indications of Movement of Structural Items on Slopes	nla
Trespassing on Slopes	small tootpath, many tree rects brush brush pile on downstram tacé
Cloughing or Erosion of Slopes or Abutments	none
hook Clope Protection - Riprap Failures	no riprap; stark till on den stram siepe
Inusual Movement or Bracking at or near Toes	nja
Grasual Embankment or Downstream Despase	seepage below that is the it is is brain slow. Small comment of staining water at the
Higher or Estar	slight popular has noch till
Founiation Drainage Features	none
Toe Irainc	none
nut mament at 1 nu stystem	none :

PERIODIC INSPECTION CHECK LIST

PROJECT Little Nugget Lake	DATE 8 / 31-78
PROJECT FEATURE Outlet works	NAME M. Larson
DISCIPLINE Geoteennical	NAME
AREA EVALUATED	CONDITION
SUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE	
a. Approach Channel	not visible
Slope Conditions	
Bottom Conditions	
Rock Slides or Falls	
Log Boom	
lebris	
Jendition of Concrete Lining	
Drains or Weep Holes	
c. Intake Structure	see notes belau
Condition of Concrete	no concrete
Stop Logo and Slots	none visible

Intake structure built of dry stene masony. Metal plate sitting on rocks, not boilted down. Valve stem externs from upstream of plate down to gate No gate mechanism on plate Plate may be removable for adding hand uncel

Transition not visible.

PERIODIC INSPECTION THECK LIST

PROJET LITTE LITTLE Nugget Lake	DATE 8:31 78
FEGUENT FEATURE OUTLET WORKS	NAME M. Larson
I IDOIPLINE Geotechnical	NAME
AREA EVALUATED	tom into
OTLET WORKS - OUTLET STRUCTURE AND SUTLET CHANNEL Stone work Reneral Jondition of Concrete	stone wall martared only in vicinity of outlet pipe Pipe 18 inch dia corrugated metal
Fast or Staining	slight rusting noted at bottom of p.p.,
.palling	none
Erosion or Cavitation	nla
Uisible Reinfording	none
any Seepage or Efflorescence	slight rust noted through water in five
Tendition at Joints	nla
Train Holes	nla
Grannel	disconnected corrected pipe conducts some if the flow to seepage pool *
Loose Rock or Trees 'ver- hanging Thannel	channel overgrown with shrubs
Condition of Discharge Channel	foir - cluttered with vegetation

* Pipe lies in eraded channel. Channel continues dawnstream past old mill house (small collapsed building)

PERIODS INSPECTED TO THESE DAME.

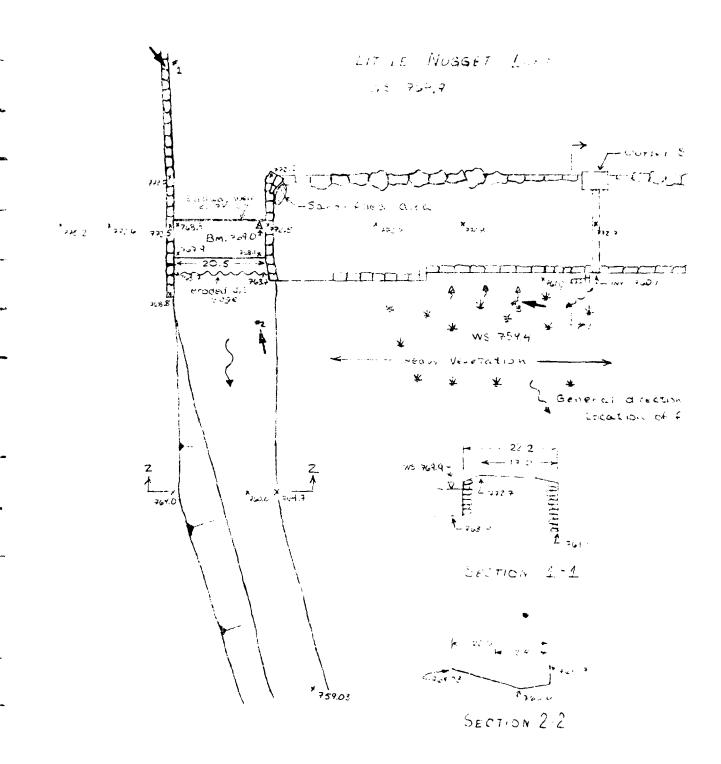
EBOBT Little Migget Lake	1 ATH 8-31-78
THE STATE FEATURE ST HWay	NAME M Larson
TINTIPLINE Hydraulie	HAME L Branagon
AREA EVALUATED	SONDITION
TLET WORKS - SPILLWAY WEIR,	mortared Stene walls some mother
a. Appraen Channel	missing some crocks
Peneral Condition	fair to good
loove book overnaming Channel	none
Trees overhanging Channel	one large tree, bushes on right wall
Floor of Approach Channel	submerged, apparently concrete
:. Weir and Training Walls	Flashboard (weadon) 115' high, 35' wide
Perenal Tendition of Tonorete *	concrete were with uspect cap, crest too and one sicpo the red one partials it is no grand of concrete open impossing the conditions.
Rust or Staining	none
Spalling	considerable amount on all slope of war
Any Vivitle Reinforcing	none; rock is soing them left thomany well,
Any Seepage or Efflorescence	passible under aspticit but covered by litter and reak datas
Drain Holes	none visible
. Discharge Channel	converges in the scepage channel about 2001 downstream **
Eneral donsition	four
Loose Rock Overhanging Channel	very little, pessible cellapse of
Trees Overhanging Channel	many small trees
Floor of diamel	netural icax pavement
ther autractions	minor streets and detries
* slight to moderate amount of mouther missing a disindeed from training wells	** House introded as left hank of Okumer about 300 feet downstream of dams

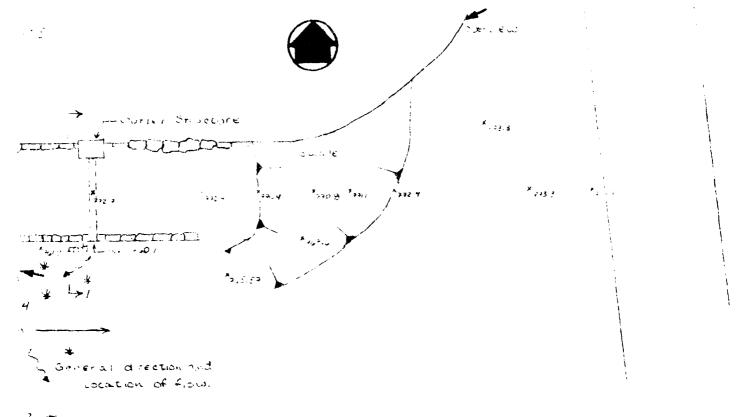
APPENDIX B

PLAN OF DAM AND PREVIOUS INSPECTION REPORTS

	Page
Figure B-1, Plan of Dam, Spillway and Sections	B-1
Figure B-2, Plan of Dike and Section	B-2
Previous Inspections (Partial Listing)	B-3
Inspection Report by Massachusetts Department of Public Works, February 1972	B - 5
Inspection Report by Massachusetts Department of Public Works, September 1973	B-7
Letter Report by Massachusetts Department of Public Works, September 1973	B - 13

LITTLE NUGGET LAKE





1000 Process

No TES!

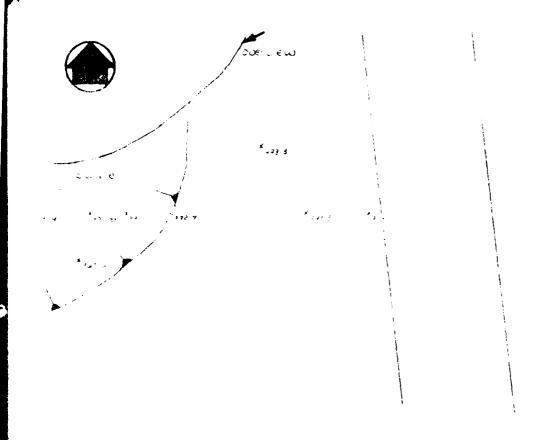
- 2 Prevarience Should based on Gost Henrike China China Chenry El 7690 (Mina)
- 2 Born Sugar taken flag to a command of
- * DENUTES SEEMANE
 * DENUTES MARSH
- " " Shows a motion or a purish protection of the time of the state o

Seale , Feet

PARTITION OF THE PARTIT

TRIBUTARY FPENCH RIVER

V

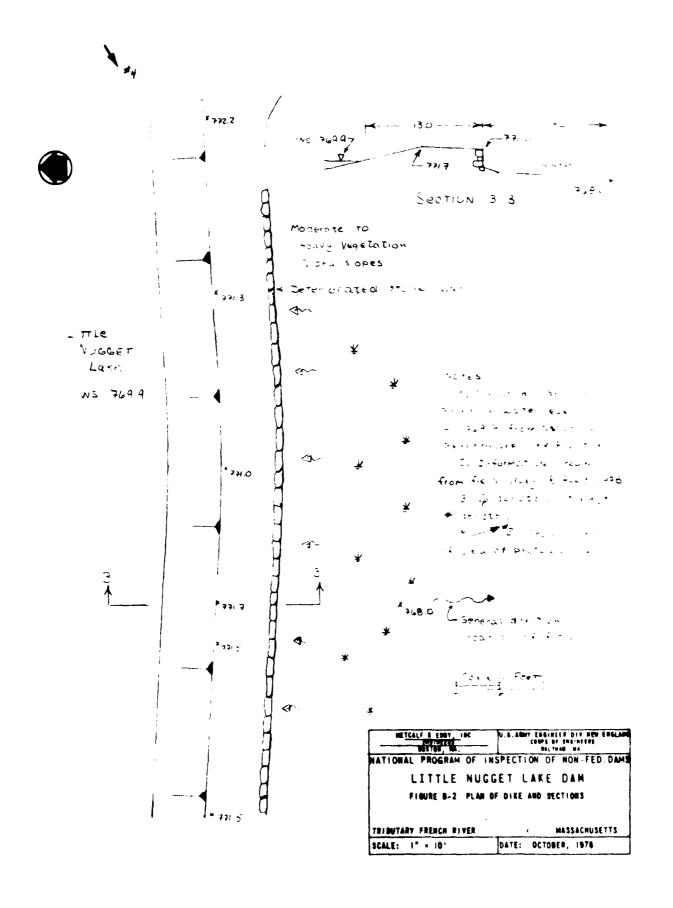


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- . E evolució. Brown Casea un cessonnes i kunch mark Tribuly Cross, El 76º D (M.S.L)
- 2 Derive our taken from the cover, A. 31, 1976
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Some Geet

METCALF & FOOV SMC.	D.S.ADWY EMGLMEEN D.V NEW ENGLAN COMPS OF THEINLERS MALTHAN, WA
NATIONAL PROGRAM OF IN	SPECTION OF NON-FED DAM
LITTLE NUGG	ET LAKE DAM
FIGURE 8-1 PLAN O	F DAM AND SECTIONS
	MA 0.0 1 000 11 00 00 00 00 00 00 00 00 00 00
TRIBUTARY FRENCH RIVER	MASSACHUSETTS
SCALE: 1" 20"	DATE: OCTOBER, 1978



Charlon-Stafford St. "Sept. 3, 1934 Marden Edwin H Perry-Garlfon, Max Flow Cut. It per Sec. Head of Fishboards Low Water """ "A Month Streams Length of Waisrshed Width """ " " Reservoir Winth "" " " Reservoir Winth "" " " Reservoir Winth "" " " " Reservoir Winth "" " " " Reservoir Winth "" " " " Reservoir Winth " " " " " How " Geral Low Water Feb. 12, 1928 " " " " " " " " " " " " " " " " " " "	TOWN OR CITY Charlton DECREE NO. 1.'C.	DECREE NO. 12 (CO. 12 C. M. 17 C. 18 C. NO.	DAM NO. 4 A . 2 L
	LOCATION NOTAL CHARTON- Statford 3	27	C. C. DOCKET NO.
	DESCRIPTION OF DAM	DESCRIPTION	OF RESERVOIR & WATERSHED
		Name of Main Stream	Little Fiver-10 Acre Pond
		Length of Watershed	
		Width " "	
		is Watershed Cuitivated	•
	-	Percent in Forests	
	=	Steepness of Slope	
		Kind of Soil	
	•	No. of Acres in Watershed	
	, ,	" " " Reservoir	
		Length of Reservoir	
		Wigth "	
	Dam designed by	Max Flow Cu. Ft. per Sec.	Town of Charles The Tilk
	" constructed by	Head or Flashboards-Low W.	
	Year constructed	High.	" Corp dellare O May 12. 1937
	1	G	ENERAL REMARKS
		Now Federal	Land Bank-Springfield.
	7	1942- Isaiah L. &	Annette S. Newman Workeste
Feb. 12, 1928' " Aug. 6. 1930 " Sept. 3, 1934 " Oct. 5, 1938 B. P. St. John Nov. 15, 1933 - L. O. Marden. 32 - E. Gorcogn & Marty	. /	Inspected. June 1	14 1940 - 1. a.M. & Lindguist
Aug 6. 1930 " " " " " " " " " " " " " " " " " " "	" Feb. 12, 1928' " "	with Mr. Hamso	of the Fee Land Som: 111611
Sept. 3, 1934 . " " " " " " " " " " " " " " " " " "	" ME 15.1933 AUP 6. 1930 "	repairs to dam.	
Oct. 5, 1938 B. P. St. John Nov. 15, 1933 - L.O. Marden. 19 - E. Gorcom & Manty	" Sept. 3, 1934 " "	Inspected. Aug 6	, 1940-LOM. Harris & Buzze.
Nov. 15, 1933 - L. O. Marden.	Measured: Oct. 5, 1938 B. P. St. John	, , , , , , , , , , , , , , , , , , ,	1. * K. M. Finlayson
39 - E. C. Gorcorn & Manty	Inspected: Nov. 15, 1933 - L. O. Marden.	1/62	12/4/5 L.K.S.
	Manured 3-23-39 - E. C. Corcorn & Manty	caned by Little	Hupper Loke Las 150sts

PREVIOUS INSPECTIONS (PARTIAL LISTING)

COPY OF INSPECTION CARD ON FILE AT THE MASSACHUSETTS DEPARTMENT OF PUBLIC WORKS, DISTRICT OFFICE, WORCESTER.

laspected : Dec. 3,1951 - 1,450

* Feb 18:1960 LOM

DUNGIL - TOWN OF CHARLTON

INSPECTED - 2/14/72 - NICHOLSON MULCHITY

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<i>₹</i>	
: INSPECTION REPORT & DATA FOR DAMS	Dam No. 54-24
Owner: Town of Charlton His Address: Board of Selectnich - Function of Dam: KECK TOWNER	Town: Larling Stream: Little River Pond: Little Hugger Lare Date: 111/17
Location & Access: 0.35 Mile N.W. on Access Rd. off Northside Turnpike (012 15 MILL WAFE) & PIGHT USGS Quad. Leicester Lat. 42°10'12" Long. 71°5702" FROM Drain.Ar.: 3.06 Sq. Mi.; Ponds: ac.; Res. Edam: GARPOF Character of D.A.:	COMPTRION DARRING
Estimated Discharge Capacity:	
General Description of Dam and Discharge Control: "STO,U = MASO,URRY WALLED DOWNSEN, RIP RAP UPSTREAM, ENTIL ELL, G. 5 YEARS OLD. 2'X 5' GATE FELTS 4"X12" FLASHIBLIAUS, Sketch (Not to Scale):	MIN, MITTABLE
N -	T = 20' 3'
MASSONARY TE-O FE 75' SATE TO SE	B-B SPILLWAY ESEVE
Remarks and Recommendations:	ACARENT DIST
Nomeras and recommendations.	- Land
Date By Comment 2/14/72 K. Thecholson	

Dam No. 3-14-54-24

Sketch (Not to Scale): A-A-DAM SIDE-VIEW -SPILLWAY SIDEVIEW Remarks and Recommendations: Comment Dam No. 3-14-54-24

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INS	PEC 1	HOIT	REPORT	: -	DAKS	AID	DESERV	LOTRS
		4011	1.6 5 00 4	_	בונתע	201.00	1,13,10	/ U I R C

	•	MOPLOTION REPORT -	DANIS AND RESERVOI	n 3
1.	Location: CII	Y/TownCharlton	Da	m No. 3-14-51-24
	Name of Dam L	tile Braint Isla	Inspecte	d by T. Televa
			Date of Inspecti	on <u></u>
2.	Owner/s: per:	Assessors	Prev. Inspec	tion 2-16-72
		Reg. of Deeds	Pers. Co	ntact
	1 Town of Ch	aniton	Chamles-	Mana Career
	Name	St. & No.	City/Town	State Tel. No.
	Name	St. & No.	City/Town	State Tel, No.
	Name	St. & No.	City/Town	State Tel. No.
_				
3.	Caretaker (If by absentee o	any) e.g. superint	endent, plant man multi owners.	ager, appointed
	Name: Board of	Selectmen	St. & No.: Town H	all Charlton
	Min Ci	narlton	State: Mass. I	el.No.:
4.	No. of Pictur	es taken <u>no</u>	me	
٥.		ard: (if dam should	·	
	1. Minor	χ	2. Noderate	
	3. Severe		4. Disastrous	
	* This rating	may change as land	use changes (fut	ure development)
6.	Outlet Contro	l: Automatic	Manual	X
		Operative	yes;	No.
	Comments: Wat	er level controlled	by flashboards a	t spillway.
				•
7.	Upstream Face	of Dam: Condition:		
		1. Good	2. Mi	nor Repairs X
		3. Major R	epairs 4. Ur	gent Repairs
. OE	Comments: Drv	stone face deterio		
		the date	TECTUE DELANCE H	The spillway

DAIL	NO:	7-71,	-51:-24
IJ & L.	NOL	,	- ,-,

8.	Downstream Face of Dam:	
	Condition: 1. Good X 2. Minor Repairs	
	3. Major Repairs 4. Urgent Repairs	
	Comments:	
9.	Emergency Spillway: None	
	Condition: 1. Good 2. Minor Repairs	
	3. Major Repairs 4. Urgent Repairs	
	Comments: 36" drawdown gate	
10.	Water Level at time of inspection: 6"st spiffweybove X below	
	top of damprincipal spillway X	
	other	
11.	Summary of Deficiencies Noted:	
	Growth (Trees and Brush) on Embankmentnone	
	Animal-Burrows and Washouts (2) 1 at gate 1 at spillway	
	Damage to slopes or top of dam Dry stone walls (upstream)	
	Cracked or Damaged Hasonry!ffncr cracking at spillway	
	Evidence of Seepagenone	
	Evidence of Piping Yes * Ses Note Page 3	
	Erosion Minor erosion downstream face	
	Leaks none	
	Trash and/or debis impeding flow at spillway outlet	ĺ
	Clogged or blocked spillway none	
	other Settlement (sinkholes)	

DAN NO. 3-14-54-24

12. Remarks & Recommendations: (Fully Explain)

Detris should be removed from spillway outlet.

Presently the dam requires minor repairs to the (two washouts and)
to the vertical face of the upstream dry wall.

Note * There is some evidence of piping and a later inspection will be made.

13. Overall Condition:

1.	Safe X
2.	Minor repairs needed upstream ince & washouts
з.	Conditionally safe - major repairs needed no
	Unsafe <u>ro</u>
5,	Reservoir impoundment no longer exists (explain)
	Recommend removal from inspection list

NITTLE DOUBLE DARK

DESCRIPTION OF DAM

	DISTRICT III				
Submitted by George D. Brodeur	Dam No. 3-11+-54-21+				
Date Sept. 4, 1973	City/Town Charlton				
	Name of Dam Little Nugget Lake				
1. Location: Topo Sheet No.	21 A				
Provide $8\frac{1}{2}$ " x 11^n in clear copy Dam clearly indicated.	y of topo map with location of				
2. Year built: Year/s of	subsequent repairs				
3. Purpose of Dam: Water Supply Recreational X					
Irrigation	Other				
4. Drainage Area: 3.C6	sq. mi acres				
5. Normal Ponding Area: 12 t	_acres; Ave. depth				
	gals.;acre ft.				
6. No. and type of dwellings locate	ed adjacent to pond or reservoir				
20 i.e. summer hom	nes, etc.				
	Max. Height				
Slopes: Upstream Face <u>Verti</u>	CEL				
Downstream Face Venti	CTE				
Width across top					
8. Classification of Dam by Materia	11:				
Earth Conc. Masonry	Spillway Stone Hasonry X				
	Other				
9. A. Description of present land					
B. Is there a storage area or f	lood plain downstream of dam which				

DAM NO. 3-14-54-24

10. Risk to life and property in event of complete failure.

No. of people ______.

No. of homes ______

No. of Businesses none

No. of industries _______. Type ______

No. of utilities none . Type _____

Railroads _____non€

Other dams 3-14-54-24 & 3-14-54-24

Other Northside Turnpike & Massachusetts. Turnpike

- 11. Attach Sketch of dam to this form showing section and plan on $8^{1}_{2}{}^{n}$ x 11" sheet.
- 12. How to Locate:

0.35 miles Northwesterly on access road off

Northside Turnpike (opposite Millward Road)

S'! EARTH + DUMPOD ROCK

T'-10' J BRUSH + Trees

EARTH AND DUMPOD ROCK

2:1

2:1

Spillways as me means to control flow of water.

September 12, 1973

Board of Selectmen Town Hall Charlton, Massachusetts

> RE: Inspection-Dam#3-14-54-24 Charlton Little Hugget Lake Dam

Gentlemen:

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An engineer from the Massachusetts Department of Public Works has inspected the above dam, owned by the Town of Charlton.

The inspection was made in accordance with Chapter 253 of the Massachusetts General Laws, as manended by Chapter 595 of the Acts of 1970.

The results of the inspection indicate that this dam is safe; however, the following conditions were noted that require attention.

- Two settlement or sinkholes, located at the gate and at the spillway, should be investigated and corrected and repaired as needed.
- The upstream face of the dry stone wall between the spillway and the gate is deteriorated and needs rebuilding.
- 3. Remove the accumulation of debris from the spillway.

We call these conditions to your attention now, before they become serious or more expensive to correct.

Very truly yours,

WILLIAM E. DONAHUE, P.E. Acting Deputy Chief Engineer

LRVaface

co: S. C. Waruzila Acting DHE#3
Dome and Feservoir Engr. Dist#3

8-13

APPENDIX C

PHOTOGRAPHS



NO. 1 UPSTREAM VIEW OF SPILLWAY



NO. 2 VIEW OF SPILLWAY FROM DOWNSTREAM TOF.



NO. 3 VIEW OF DOWNSTREAM WALL OF DAM



NO. 4 VIEW OF DIKE

APPENDIX D

HYDROLOGIC AND HYDRAULIC COMPUTATIONS

LITTLE NUGGET LAKE

Project Nat. Review of Non Fed. Dams Acct. No. 6036 Page 1 of 6 Subject Worcester Mass. Area Comptd. By LEB Date 10/2/78

Detail LITTLE NUGGET LAKE DAM Ckd. By MLL Date 10/10/78

I Test Flood, 100 year storm & Storage Functions

1 - Total Orainage Area - 3.05 mi

2- Pond(s) Area: 0.02 + 0.01 + 0.05 = 0.08 mi Swamp(s) Area: 0.16 + 0.15 = 0.31 -Total Area Pond(s) & Swamp(s): 0.39 -

70 Ponds & Swamps = 0.39 = 13%

3 - El. 1075 - 769 = 3.0%.
10400 ft = 3.0%.
El. 1075 - 764 = 2.0%.
15200 ft = 2.0%.

} Say Ave Slope = 2.5%

4-Using C. of E. Curves for Peak Flow Ruter & above quide values the Peak Flow Rate was estimated to be between Kolling and "Flat and Coastal", and taken at 1350 c.f.s./mi

Due to low dan height use 1/2 MPF

5- Test Flood Inflow = = (1350) 3.05 = 2060 afs

6- Pond Storage
The pond area is 0.02 sq, mi, at elev.
Based on a const area, storage increases
at 12.8 ac. feet per foot of depth increase.
Although storage /foot will increase with
depth, the variation is not significant for
this study.

7- Storage Functions are based on Pour = Pin[1- Sour]

Sout = Storage Vol. in Reservoir related to Sind Cout
in terms of inches of rain over the draining anca

S(in Inches) = 12 D (0.02) = 0.08 D g R = 5 hr rain of 5 mm

D = Storage Depth (2000 Spriman) on reservoir in fee

8 - Storage Functions: (==) D=0'@Pond =1.769

FT= = 2060 - 217 5 = 2060 - 17.4D

Project Nat Fourcis of Almis Jams Accorno 6036

Subject Worceston, Ma. Area compid by LEB Dute 7/25/79

Detail LITTLE NUEGET LAKE DAM Chd By MLL Dute 10/10/79

The Dam Discharge Ratings

A-Broad Crested Spillway Pos = CLH 1.5 [Ref. pp. 360-362]

(See below)

C= 3.27 + 0.4 H/h, L= L'-0.1 N H

H=Physical Water Head on CREST (hy not included)

h= Weir Height, L'= Measured Crost Length

Assumptions

For Floods or feak Flows, H ~ 0.5 : C: 3.47 L= 90% L'

$$P_s = 3.12 L H^{3/2}$$



B-Flow over Creed of Dam - g= 3.475 [y+h] (H') [see below g= Disch /f- of width
H'& h' as defined above; y = h'+ H'

Assumptions

For Floods (flow over dam crest)

H'= 6 h' [note h'= h+H in Item Dalsoue]

 $y = \frac{7}{6}h' \notin \left[\frac{y}{y+h'}\right]^{h'} = \left[\frac{\frac{7}{6}h'}{\frac{1}{2}h'}\right]^{h} = 0.734$

Pc = 2.55(H') Apply to Cuest in steps where levels are roughly const.

[Ref. : "Open Channel Hydraulics" - V.T. Chow]

	3 01 4.
Subject Worcester Mass. Area Comptd By LEB Date	9/25/78
000), 00	10/10/78

I Dam Dischange Rating - Cont.

C- Four areas of discharge

1- Spillway: Qs = 3.12 (20.5') H"5; Crest @E1.769.0*

2- Emergency Spillway (Swale): QES= 2.55 (20') H" Crest @ El. 770.8 ±

3- West Dike Crest: Qwo = 2.55(80') H"; Crest@E1,771.5 ±

4 - Main Dam Crest: Quo = 2,55(150')H"5; Crest@El.772.7±

D- Dischange us Pond Elev.

Pond Elevi	Q5*	Q _F s	Pwo	QMD	Total Disch(c.f.s.)
770 771 772 772.5 773.5 774	04 1832 4512 5015	4736642			04 185 471 736 1116 1688 2380

* Assume 15" + flash board has failed

E - Outlet Pipe Discharge

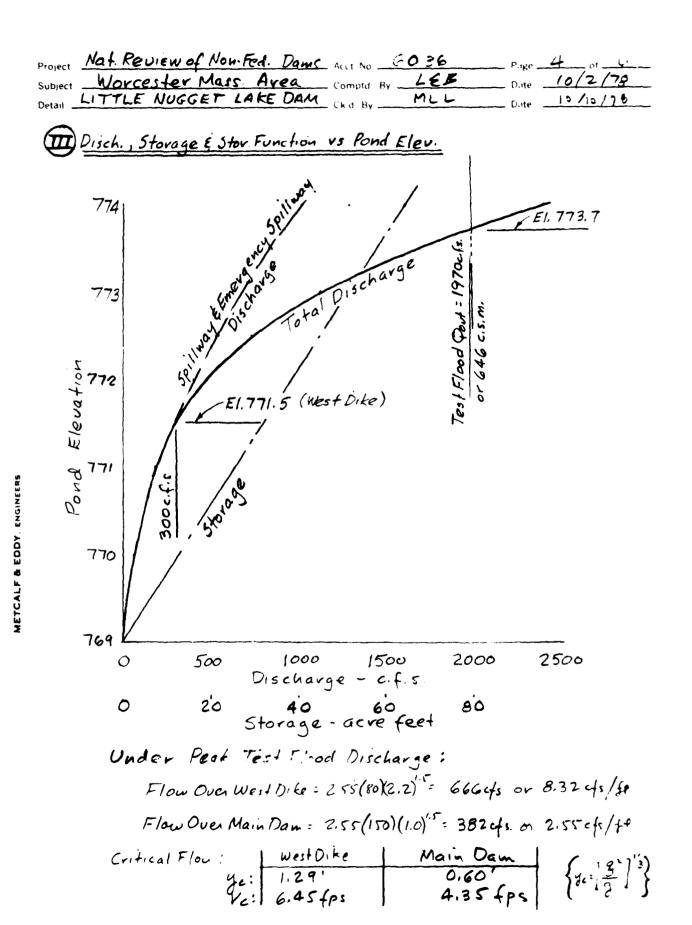
Pond E1. 770 ± , & outlet 760.5± , Net Head = 9.5'
Ignore pipe friction, Head Loss = 1.0 hy@exit +0.5 hy@ent.

1.15 /2g = 9.5; V = 20 fps - Use 18.0 fps to allow for frict.

Pipe Size 18"c.u.p , Avea = 1.767 ft". Qout = 32 cfs.

Time to Drain 1' = 0.02(640) 43560 = 4.8 hr.

32 cfs from dranger axa = 10.5 c.s.m.



Project Nat, Review of Nou-Fed. Dams

Subject Worcesfer Mass Area Comptd By LEB Date 10/2/78

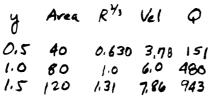
Detail LITTLE NUGGET LAKE DAM Ckd By MLL Date 15/10/78



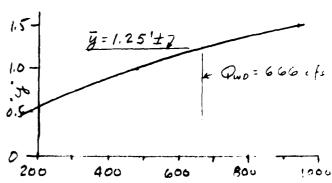
West Dike Discharge Channel

Flow over the West Dike would flow thru a swampy record channel. From USGS this channel slope is 9'in 350' or .0257 For Vel. use: $V = \frac{1.19}{1.04} R^{4/3} (.0257)^{1/2} \approx 6.0 R^{4/3}$

Say Channel 80' wide & flat R=y = Depth Flow



Ave Flow Vel = 7.0 fps ±



Project Nat Reviser of Non Fed Dams Acct No - 6036 Page Worcester Mars, inca Compid By LEB LITTLE NUGGET LAKE DAM MLL Failure of Dam Peak Failure Flow: Pond Elevation - 772 ± Toe Elevation - 761 ± Dam Length Subject to Breaching = 140' Wo = 40% (140) = 56' QR = 1.68 Wo (Yo)"= 1.68(56)(11)"= 3400 its. Storage Volume Released: Storage Above Spillway: 3 (0,02) 640 = 38.4 acre ft Storage Below Spilway: 1/3 (B) (0.02) 040 = 34.1 "
Storage Below Spilway: 1/3 (B) (0.02) 040 = 34.1 "
72.5 5 = Total Storage = Channel Hudraulies: 300' 5= 850 = .0235, 51 = 0.153, n = 0.10 A= 15 y', P-30y, R= 1/2, Vel= 2.285 R 1/2 R43 V .630 1.44 22 (y : 6.6'+ 2.28 137 @ 3400ck 3 135 1.31 3.00 404 A= 653 ft2 4 240 1.59 363 870 5 375 1.84 4.20 1578 6 540 2.08 4.75 2566 7 735 231 5.27 3871 3000 V, = 653 x 700' = 10.5 ac. ft., OR = 3400(1-105) = 2900cfs, y = 6.3 ft Az= 595A', Ave A==(653+595)=624A', g=6.45', Q= 3100 cfr Note: 700' is distance to "critical" house below dom

Time to Drain!

43560 (72.5)
3600 (1/2.5) = 0.52 Hours = 31 minutes

APPENDIX E

INFORMATION AS CONTAINED IN THE NATIONAL INVENTORY OF DAMS

